

PLANNING PROPOSAL TRAFFIC AND PARKING IMPACT ASSESSMENT OF MIXED USE RESIDENTIAL & COMMERCIAL AT 167 HUME HIGHWAY, CHULLORA



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1 INTRODUCTION

M^cLaren Traffic Engineering (MTE) was commissioned by *Iris Capital* to provide a Planning Proposal Traffic and Parking Impact Assessment of the proposed redevelopment of The Palms, Chullora to a Mixed Use Residential & Commercial development. The site is located at 167 Hume Highway, Chullora. The proposed plans are reproduced in **Annexure A** for reference.

1.1 Description and Scale of Development

The proposed planning proposal consists of the following scale relevant to this report:

- Hotel floor area of 970m² in Building A
- 18 serviced apartments in Building A
 - 3 x 1 bedroom apartments
 - 6 x 2 bedroom apartments
- 63 units within Building B
 - 12 x 1 bedroom apartments;
 - 47 x 2 bedroom apartments;
 - 4 x 3 bedroom apartments.
- 32 units within Building C
 - 24 x 2 bedroom apartments;
 - 8 x 3 bedroom apartments.
- 54 units within Building D
 - 1 x 1-bedroom apartment
 - 47 x 2-bedroom apartment
 - o 6 x 3-bedroom apartment
- On-site car parking within basement parking levels for both the commercial and residential portion of the development. Basement parking will be separated for the commercial parking and residential parking.
- On-site loading bay to be utilised for both delivery and waste collection located adjacent to the proposed hotel, facilitating up to a 12.5m length Heavy Rigid Vehicle via a forward entry / forward exit from the site.
- Vehicular access from the Hume Highway only, via two newly created two-way driveways along the Hume Highway. The most northern proposed two-way driveway will residents only, with the southern driveway for services and hotel patrons.

1.2 State Environmental Planning Policy (Infrastructure) 2007

The proposed development does qualify as a development with relevant size and/or capacity under Clause 104 of the SEPP (Infrastructure) 2007. Accordingly, formal referral to the Roads and Maritime Services (RMS) is necessary and Canterbury-Bankstown Council officers can refer this proposal accordingly.

1.3 Site Description

The subject site, officially identified as Lot 402 DP 631754, is irregular in shape with frontage to Hume Highway only. The boundary length along the Hume Highway is 171.3m in length

and currently has four (4) vehicular driveways onto the Hume Highway. The site is currently zoned B6 – Enterprise Corridor as per Bankstown Council Local Environmental Plan (LEP).

Internally, the existing site has three (3) structures consisting of a restaurant, The Palms Hotel premises and associated units and a storage brick building. There are currently 166 line-marked car parking spaces throughout the site, shared between the Palms Hotel and the restaurant. The existing GFA of these buildings are 1,300m² GFA.

The site is bounded by low density residential to the eastern side of the Hume Highway, whilst the western side of the Hume Highway consists of large bulky goods (Masters retail hardware) and warehouse / industrial complexes.

1.4 Site Context

The site location is shown on aerial imagery and a map in Figure 1 & Figure 2 respectively.



Site Location

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FIGURE 1: SITE CONTEXT – AERIAL PHOTO





Site Location



2 EXISTING TRAFFIC AND PARKING CONDITIONS

2.1 Road Hierarchy

Hume Highway has the following characteristics within close proximity to the site:

- RMS Classified MAIN Road (No. 638);
- Approximately 25m in width (including a medium strip) facilitating three lanes in both directions;
- Signposted 70km/h carriageway;
- No Parking signage along both sides of the road;
- Approved 25/26m B-Double truck route.

Muir Road has the following characteristics within close proximity to the site:

- Unclassified Collector Road;
- Approximately 19m in width (including a medium strip) facilitating two lanes in both directions;
- Signposted 60km/h carriageway;
- Unrestricted kerb side parking where available along both sides of the road;
- Approved 25/26m B-Double truck route.

Tennyson Road has the following characteristics within close proximity to the site:

- Unclassified LOCAL road;
- Approximately 10m in width facilitating two-way passing and kerbside parking;
- No speed limit signposted 50km/h applies;
- Unrestricted kerbside parking permitted along both sides of the road with sections of 3-hour restricted kerbside parking on both sides of the road.

Peter Crescent has the following characteristics within close proximity to the site:

- Unclassified LOCAL road;
- Approximately 7m in width facilitating two-way passing and kerbside parking;
- No speed limit signposted 50km/h applies;

• Unrestricted kerbside parking along the east side of the road with "No Parking" signage along the west side of the road.

2.2 Existing Traffic Management

- Signalised intersection of Hume Highway & Muir Road
- Left in/ Left out "Give Way" junction of Hume Highway & Tennyson Road
- Priority controlled intersection of Tennyson Road & Peter Crescent
- No vehicular access from Hume Highway into Hillcrest Avenue / Cardigan Road

2.3 Existing Traffic and Parking Environment

Hume Highway currently carries in the range of 58,000 two-way daily vehicles north of Waterloo Road and some 57,000 two-way daily vehicles south of Brunker Road. Historically, traffic volumes north of Waterloo Road appear to have remained constant, with a slight increase in the average daily vehicles during 2017 and 2018. Whilst traffic volumes south of Brunker Road appear to have gradually decreased, between 2015 and 2016, compared to the previous years and increased in 2017 and 2018 (back to the average two-way traffic flow in years 2013 and 2014).

An intersection survey was undertaken at the signal controlled intersection of Hume Highway & Muir Road on Friday 19th August 2016. The survey sheets are provided in **Annexure B** for reference.

Existing intersection performances have been assessed using SIDRA INTERSECTION 7.0. The analysis is summarised in **Table 1** below with detailed outputs reproduced in **Annexure C** for reference.

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/vehicle)	Level of Service ⁽³⁾	Control Type	Worst Movement	
	EXISTING PERFORMANCE						
Hume Highway /	AM	0.58	17.1	В	Signal	N.A	
Muir Road	РМ	0.57	18.6	В	Signal	N.A	

NOTES:

(1) Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.

(2) Average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.

(3) Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets. As shown above, the intersection of Hume Highway & Muir Road is operating at Level of Service (LoS) "B" which reflects satisfactory operation with spare capacity and minimal delays.

In addition to the intersection survey being undertaken, driveway movements were also recorded along the site's Hume Highway frontage, the survey sheets are provided in **Annexure B** for reference. During the morning peak hour of 8:45 to 9:45 there were 12 two-way traffic movements. During this time period there were 25 vehicles parked on-site which represents a peak hour generation rate of 0.5 trips per car space in use. The evening peak hour of 6:00-7:00pm found 47 two-way traffic movements. During this time period there were 69 vehicles parked on-site, which represents 0.68 peak hour movements per car space in use. The two-way traffic movement during the morning peak hour was 26% of the evening peak hour.

2.4 Gap Summary

A gap assessment was conducted on 30th February 2017 during the peak AM and PM time period. The gaps surveyed the number of gaps over the 5 traffic lanes, including the right hand turn lanes as shown in **Figure 3** below. The survey results have been reproduced in **Annexure B** for reference.



FIGURE 3: LOCATION OF GAP SURVEY

The results of the gap survey are summarised in **Table 5** below.

Peak Time	Peak hour	Minimum Gaps from 5 to 8 seconds	Minimum Gaps greater than 8 seconds	Minimum gaps within a one hour period
AM	8:15am – 9:15am	98	49	147
PM	4:00pm – 5:00pm	73	70	143

TABLE 2: GAP ASSESSMENT

As shown in **Table 2** above there is a minimum of **147** and **143** gaps in the AM and PM period respectively. This shows that there are adequate gaps in traffic to allow vehicles to exit the site. Furthermore, there are additional routes for vehicles to travel to head north. If vehicles cannot find an acceptable gap to turn right at Muir Street, they can easily travel south to the intersection of Brunker Road / Hume Highway / Rawson Road where they can turn right.

It is relevant to note that the proposed driveway located at the end of the right turning lanes into Muir Road will be solely used by visitors to the hotel, which typically have their peak traffic movements outside the peak commuter AM and PM periods.

2.5 Public Transport

The subject site has access to existing bus route 925 provided by Transdev NSW which runs through East Hills to Lidcombe via Bankstown. The nearest bus stop is located along the Hume Highway within a 90m walking distance of the site. The 925 bus route provides access to Lidcombe Train station and East Hills Train Station. Bus Route 925 operates every 30 minutes during peak AM and PM weekday commuter periods and every 1-hour outside peak commuter periods. Hourly services are provided on Saturdays and Sundays from 6am to 11pm. **Figure 3** shows the bus route for 925 relative to the location of the development



SITE LOCATION

FIGURE 4: BUS ROUTE MAP

2.6 Future Road and Infrastructure Upgrades

From The New City of Canterbury-Bankstown Council's Development Application tracker and website, it appears that there is no future planned road or public transport changes that will affect traffic conditions within the immediate vicinity of the subject site.

3 PREVIOUS APPROVALS

The site has previously been approved to construct a drive-through liquor shop under Development Application (DA) 11153/2010. The development consisted of the following:

- 122m² of display area
- 58m² cool room area
- Drive through bay for 5 vehicles
- Drive through express lane for 5 vehicles
- 2 parking bays for customers
- 3 parking bays for staff
- 1 disabled parking space

The application identified removal of 23 car parking spaces from the overall provision of 126 car parking spaces. Conclusions of the *Traffix* assessment (dated November 2010) is that:

- Existing site parking demand was 61 car spaces out of 126 car parking spaces
- Under the proposal, 103 car spaces (60 hotel, 24 restaurant and 22 motel) were retained.
- Deliveries to the proposal to be made by a 10.4m rigid truck
- Proposed traffic generation of 130 vehicles in the peak hour (65 in; 65 out)
- Adopted a passing trade of 90%, such that only 10% of the generation is classed as 'new' trips to and from the development.

4 PARKING ASSESSMENT

4.1 Council Parking Requirement

The site is within the newly formed Canterbury-Bankstown Local Government Area, however the planning controls of the Bankstown Council remain in place.

As such, the Bankstown Council Development Control Plan 2015 (BCDCP 2015) is the relevant control document stipulating, amongst other things, Council's car parking requirement. The BCDCP 2015 states the following car parking rates applicable to the proposed development:

Part B5 – Parking

Residential Flat Building, In Zone R4, B1, B2 & B6 1 car space per 1 bedroom dwelling; or 1.2 car spaces per 2 bedroom dwelling; or 1.5 car spaces per 3 or more bedroom dwelling; and 1 visitor car space per 5 dwellings

Hotel or Motel Accommodation 1 car space per unit; and 1 car space per 2 employees

Restaurants – Outside of Centres 0.15 car space per square metre of total dining or bar area. Total dining bar area means all of those parts of a restaurant, catering or reception centre where customers order or are served food or drink, and includes waiting areas.

Drive-in Liquor Store Off-street car spaces for "browse-room" customers' and 1 car space for each employee Where customers park and leave their vehicles to purchase liquor, a drive-in liquor store may be considered as a shop. Under these circumstances, 1 car space per 40m² of gross floor area will be required.

BCDCP 2015 states that the total number of car parking spaces required for a development is to be rounded down if the calculation results in less than half a space, or rounded up if the calculation results in equal or more than half a space.

Based on BDCP 2015 car parking rates, the car parking requirement is summarised in **Table 2**.

Use	Scale	Rate	Requirement	Provision
Residential Flat	6 x 1 bed	1.0 per unit	6	6
Building-	12 x 2 bed	1.2 per unit	14.4	24
Building A	Sub-Total		20.4	30
Desidential Flat	12 x 1 bed	1.0 per unit	12	12
Residential Flat	47 x 2 bed	1.2 per unit	56.4	76
Building-	4 x 3 bed	1.5 per unit	6	6
Building B	Sub-Total		74.4	94
Residential Flat	24 x 2 bed	1.2 per unit	28.8	46
Building-	8 x 3 bed	1.5 per unit	12	12
Building C	Sub-Total		40.8	58
Residential Flat	1 x 1 bed	1.0 per unit	1	1
	47 x 2 bed	1.2 per unit	56.4	67
Building-	6 x 3 bed	1.5 per unit	9	9
Building D	Sub-Total		66.4	77
Residential Flat Building	16/ units		33.4	35
Sub-Total	-	-	235.4 (235)	294
Hotel	970m ² Dining/Bar	0.15 per 1m ²	145.5 (146)	TBD
Total	-	-	381 (235 +146)	294 + Hotel

TABLE 3: BDCP CAR PARKING REQUIREMENT

As shown above, the site requires **235** residential spaces including **33** visitor spaces. The site provides **294** residential spaces including **35** visitor spaces, exceeding Council DCP car parking requirement for the residential component of the site.

The proposed hotel requires a **146** car parking spaces based on Council's DCP car parking requirement. It is relevant to note the following with respect to parking for the existing hotel on-site to determine the appropriate parking controls for the hotel portion of th development.

4.1.1 Hotel Parking

The parking surveys undertaken show current underutilisation, with a peak of some 69 vehicles on-site with an existing hotel restaurant GFA of 1,300 m². This equates to approximately 5.3 spaces per $100m^2$ GFA. Therefore, based upon the $970m^2$ GFA, this results in **52** car parking spaces (this most likely over estimates the rate as it includes the restaurant uses as well).

Adopting the 2010 consent required 113 spaces. This equates to a rate of 8.7 spaces per 100m². Based upon the proposed 970m² results in **84** spaces rounded down (this most likely over estimates the rate as it includes the restaurant uses as well). Hence based upon the above and Council's DCP car parking requirements adopting a conservative approach, some **84** car parking spaces would be sufficient for the parking demand of the hotel development. The provision parking for the hotel will be detailed during DA stage.

4.2 Bicycle & Motorcycle parking Requirements

BDCP 2015 Part 5- Section 5.18 states that "*Council may require development to provide appropriate bicycle parking facilities either on-site or close to the development.*" Council's DCP does not specify a rate of provision for any land use.

Reference is made to Austroads Guide to Traffic Engineering Practice Part 14 – Bicycles (1993) which outlines the following bicycle provisions:

Residential Units 1 bicycle space per 4 units for residents 1 bicycle space per 16 units for visitors

Hotel

bicycle space per 25m² bar floor area for employees
 bicycle space per 100m² beer garden area for employees
 bicycle space per 25m² bar floor area for customers
 bicycle space per 100m² beer garden area for customers

Restaurant

1 bicycle space per 100m² of public area for employees 2 bicycle spaces for customers

It should be noted that the above bicycle rates provide a guide to the number of bicycle parking spaces which could be provided for various land uses.

Based on the above rates, the number of bicycle spaces required as a guide for the development is summarised in **Table 4** below.

Use	Scale	Requirement		
		Resident / Employee	Visitor / Customer	
Residential	167 units	41.75 (42)	10.4 (10)	
Pub/ Resturant	970m ²	10 ⁽¹⁾	2 ⁽¹⁾	

TABLE 4: BICYCLE PARKING GUIDELINE

Note 1) Adopt restaurant rate as no beer garden for hotel and due to large on-site residential component

As shown above, based on the Austroads guideline for bicycle parking, the site could ideally provide **42** residential spaces, **10** residential visitor spaces, **10** employee spaces and **2** visitor spaces for the proposed hotel. This parking requirement is a guide for Council and is not strictly required for the subject development.

Council's DCP does not outline any parking rates for motorcycle parking and as such the site does not require the provision of this facility. It is envisaged that some motorcycle parking will be provided during detailed DA stage.

4.3 Servicing & Loading

BCDCP 2015 Section 5 requires the following with respect to loading and unloading facilities:

Mixed use development must provide appropriate loading/unloading or furniture pick-up spaces. If no provision is made for the facilities, development applications must provide justification why they are not necessary.

Where rear lane access is not available and the commercial/retail gross floor area of a building is greater than 500m², Council requires:

- a) At least on off-street parking space for delivery/service vehicles; and
- b) Additional off-street parking spaces or a loading dock depending on the size, number, and frequency of delivery/service vehicles likely to visit the premises

The design of loading docks must:

- a) Be separate from parking circulation or exit lanes to ensure safe pedestrian movement and uninterrupted flow of other vehicles in the circulation roadways;
- b) Allow vehicles to enter and leave an allotment in a safe manner; and
- c) Have minimum dimensions of 4 metres by 7 metres per space

Bankstown Council's Waste Education Officer has advised that Bankstown Council's waste collection vehicles is as per the following specifications:

- Rear loader
- Total length of 12.5m
- Turning radius of 12.5m
- Sweep circle of 27.8m
- Headroom clearance of 4.5m

Council's waste vehicle specifications are similar to the Australian Standard 2890.2:2002 specification for a 12.5m Heavy Rigid Vehicle (HRV) which is as follows:

- Total length of 12.5m
- Turning radius of 12.5m
- Outer body swept path of 27.8m
- Headroom requirement of 4.5m

A central delivery and waste collection area is provided on-site adjacent to the hotel. The area is designed to accommodate an HRV via a forward entry / forward exit onto The Hume Highway.

4.4 Disabled Parking

BCDCP 2015 Part B5 requires car parking spaces for people with disabilities to be provided at a rate of 1 space per 100 car spaces. The provision of disabled car parking will be determined during detailed DA stage for the hotel development. With respect to residential units, BCDCP 2015 Part B1, Section 9 requires the following for residential flat buildings, serviced apartments and shop top housing:

"Residential flat buildings, serviced apartments and shop top housing with 10 or more dwellings must provide at least one adaptable dwelling plus and adaptable dwelling for every 50 dwellings in accordance with AS4299 – Adaptable Housing."

Based on the provision of 167 units, the development is required to provide 5 adaptable units (1 + 167/50) compliant with AS4299. This therefore results in a required provision of 5 disabled car parking spaces for residents.

4.5 Car Park Design & Compliance

Car parking areas shall be designed in accordance with AS2890.1:2004, AS2890.6:2009 and AS4299:1995 where applicable. The notable design criteria of these standards are as follows:

- Residential car parking spaces shall measure a minimum of 2.4m in width by 5.4m in length;
- Residential visitor car parking spaces shall measure a minimum of 2.5m in width by 5.4m in length;
- Aisle widths for User Class 1/1A shall be a minimum of 5.8m;
- A 1.0m blind aisle extension is required for blind aisles;
- An additional 300mm clearance on top of the base parking dimensions is required to obstructions and walls greater than 150mm in height;
- Disabled parking spaces shall measure 2.4m wide by 5.4m in length, with an adjacent shared zone of the same dimensions;
- Residential disabled (adaptable) spaces shall measure 3.8m in width by 5.4m in length;
- Headroom for passenger cars shall be minimum 2.2m in all locations, increasing to 2.5m above disabled parking spaces and shared zones.

Loading areas and bays shall be designed in accordance with AS2890.2:2002. Notably, the following design criteria should be met:

- Minimum Loading Bay Widths
 - HRV 12.5m x 3.5m
- Headroom of 4.5m above loading areas and all vehicular path of travel;
- Loading area grade is to be no greater than 4% in any direction;
- Access ramps to loading area
 - HRV Maximum grade of 15.4% with a grade changes of 6.25% over 7m transitions.

A detailed compliance review and swept path testing will be undertaken during DA stage.

5 TRAFFIC ASSESSMENT

Traffic generation has been based upon those rates specified in the RMS *Guide to Traffic Generating Developments* (October 2002) with due consideration also given to the updated data from the RMS (RMS Technical Direction TDT 2013/04).

5.1 Traffic Generation

The traffic generation for the residential and commercial component is based upon the following:

0.29 Trips per Unit	
0.5 to 0.68 Trips per Car Space	

RMS Guide to Traffic Generating Developments Survey of Existing Site

The estimated traffic generation level for the development is based upon the RMS ""*Guide to Traffic Generating Developments*", which assumes a worst case of a high proportion of private vehicle trips. The traffic generation is summarised in **Table 5** below.

		Traffic	Peak Hour Traffic Generation		
Land Use	Scale	Generation Rate	AM	РМ	
Residential	167 units	0.29 / unit ⁽¹⁾	49 (10 in; 39 out)	49 (39 in; 10 out)	
Commercial	84 car spaces ⁽³⁾ -	0.5 / car space ⁽²⁾	42 (21 in; 21 out)	-	
		0.68 / car spaces ⁽²⁾	-	57 (29 in; 28 out)	
Sub Total			91	106	
Less Existing			-12 ⁽²⁾	-47 ⁽²⁾	
Net Total			79 (25 in; 54 out)	59 (44 in; 15 out)	

TABLE 5: TRAFFIC GENERATION OF SITE

Notes 1) assumes residential split of 80% outbound and 20% inbound during the AM peak period and vice versa during the PM peak

2) Assumes a 50/50 split of commercial trips as entry / egress

3) An estimate of parking requirements based upon the existing 2010 approval of the site and traffic surveys

As summarised by **Table 5** above, the forecast traffic generation is **91** two-way trips in the morning peak hour and **106** two-way trips in the evening peak hour.

When taking into consideration the existing site generation, the net increase on the surrounding road network is **79** vehicle trips in the morning and **59** vehicle trips in the evening.

5.2 Traffic Assignment

As the site is restricted to left in / left out access only, all outbound vehicles will travel through the signalised intersection of Muir Road / Hume Highway.

5.3 Traffic Impact

The traffic generation outlined in **Section 5.1 & 5.2** above has been added to the existing traffic volumes recorded. SIDRA INTERSECTION 7.0 was used to assess the intersections performance. The purpose of this assessment is to compare the existing intersection operations to the future scenario under the increased traffic load. The results of this assessment are shown in **Table 6** below:

Intersection	Peak Hour	Degree of Saturation ⁽¹⁾	Average Delay ⁽²⁾ (sec/vehicle) (ISTING PERFO	Level of Service ⁽³⁾	Control Type	Worst Movement		
	T	EA	ISTING FERFU					
Hume Highway /	AM	0.58	17.1	В	Signals	N.A		
Muir Road	PM	0.57	18.6	В		N.A		
	FUTURE PERFORMANCE							
Hume	AM	0.60	17.8	В	Cignolo	N.A		
Highway / Muir Road	РМ	0.58	18.6	В	Signals	N.A		

TABLE 6: INTERSECTION PERFORMANCES (SIDRA INTERSECTION 7.0)

NOTES:

(1) Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.

(2) Average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.

(3) Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets.

As shown in **Table 6** above, the performance of the key intersection of the Hume Highway / Muir Road remains unaltered under the future scenario. The existing LoS has been retained with minimal delays and additional capacity maintained.

5.4 Gap Assessment

The southern access driveway for the development is located such that vehicles can manoeuvre into the right turn lane on the Hume Highway. A gap assessment was conducted to support this movement; the results of the survey are summarised in **Section 2.4**. **Table 7** outlines the minimum number of gaps and the corresponding peak hour generation.

Peak Time	Peak hour	Minimum Gaps from 5 to 8 seconds	Minimum Gaps greater than 8 seconds	Total Gaps > 5 Seconds	Peak Hour Traffic Generation and split
AM	8:15am – 9:15am	98	49	147	42 (21 in, 21 out)
PM	4:00pm – 5:00pm	73	70	143	57 (29 in, 28 out)

TABLE 7: GAP ASSESSMENT

As shown above, the number of acceptable gaps for vehicles to enter/exit the Hume Highway in the AM peak time is **147** this corresponds to a peak hour traffic generation of **42** (21 in, 21 out). Based on the **21** vehicles exiting the site there are sufficient gaps within the AM period for vehicles to manoeuvre into the right turn lane on the Hume Highway. Similarly, during the PM peak period the minimum gaps surveyed during the PM period was **143**, corresponding to a peak traffic generation of **57** (29 in, 28 out). Based upon the **28** vehicles exiting the site there are sufficient gaps to manoeuvre into the right turn lane on the Hume Highway.

It is relevant to note that the proposed driveway located at the end of the right turning lanes into Muir Road will be solely used by visitors to the hotel, which typically have their peak traffic movements outside the peak commuter AM and PM periods, namely 7:00 - 9:00 and 4:00 - 6:00 pm. This is further supported by the existing survey of the site which had its peak traffic generation occur at 8:45 to 9:45 am and 6:00 - 7:00 pm.

Furthermore, during the AM and PM period not all vehicles exiting the site will be undertaking this movement, as a result the driveway location is fully supportable in terms of the safety aspects and traffic flow.

5.5 Residential Amenity

The site fronts and has access only to the Hume Highway, a classified roadway. This state road is a major arterial carriageway that is not sensitive to residential amenity considerations.

6 CONCLUSION

In view of the foregoing, the subject planning proposal (as depicted in **Annexure A**) is fully supportable in terms of its traffic and parking impacts. The following outcomes of this traffic assessment are relevant to note:

- Council's DCP requires the provision of 235 car spaces for the residential component and 146 commercial spaces. The site provides 294 residential spaces satisfying Council's DCP car parking requirement for the residential component and it is expected that at least 84 car parking spaces will be provided for the commercial component of the site. The provision of 84 car parking spaces is supportable based upon the previous consent condition the shortfall of 62 spaces from Council's DCP will not have a detrimental impact on the surrounding land uses and it is not envisaged that any over flow will occur onto the surrounding streets. The exact provision of parking for the hotel development will be detailed further during DA stage.
- Council's DCP does not outline any parking rates for motorcycle parking and as such the site does not require the provision of this facility. It is envisaged that motorcycle parking will be provided within the basement car park, which will be detailed during DA stage.
- Based on Council's DCP requirements, the residential component requires five (5) disabled parking spaces. The commercial portion of the development requires one disabled car parking spaces for every 100 car spaces provided. The provision of the commercial disabled car parking spaces will be detailed during DA stage.
- A central delivery and waste collection area is provided on-site. The area will be designed to accommodate a 12.5m length HRV.
- Based on the Austroads guideline for bicycle parking, the site should provide 42 residential spaces, 10 residential visitor spaces, 10 employee spaces and 2 visitor spaces for the proposed hotel. This parking requirement is a guide for Council and is not strictly required by the development.
- Car parking areas shall be designed in accordance with AS2890.1:2004, AS2890.6:2009, AS2890.2:2002 and AS4299:1995 where applicable.
- The forecast traffic generation is 91 two-way trips in the morning peak hour and 106 two-way trips in the evening peak hour. When taking into consideration the existing site generation, the net increase on the surrounding road network is 79 vehicle trips in the morning and 59 vehicle trips in the evening. The impact of some 79 (25 in, 54 out) and 59 (44 in, 15 out) vehicle trips on the surrounding intersections remain unaltered under the future scenario. The existing LoS has been retained with minimal delays and additional capacity maintained.



ANNEXURE A: PROPOSED PLAN

(Sheet 1 of 2)



ANNEXURE A: PROPOSED PLAN

(Sheet 2 of 2)

(SHEET 1 OF 6)

Curtis Traffic Surveys	Turning movement count			Liverpool Rd	
JOD:	160805mcl (16_408)			280	2106
Day, date	19/08/16	Peak Hour Volumes	222	*	
Location:	Hume Hy & Muir Rd	Multi- Dat	153		
Weather:	Fine	► Muir Rd		72	1467
Client:	McLaren Traffic Engineering				

	From Hume H'y	north From	Muir Rd	From	Hume H'y south		
Time Period	through rig	ght left	right	left	through	Total v	ehicles Peak
06:00 to 06:15	215	31	28	14	17	504	809
06:15 to 06:30	317	68	32	22	17	<mark>460</mark>	916
06:30 to 06:45	254	51	33	25	19	514	896
06:45 to 07:00	263	42	30	26	24	<mark>528</mark>	913
07:00 to 07:15	390	57	55	24	17	513	1056
07:15 to 07:30	420	48	40	27	18	403	956
07:30 to 07:45	420	65	38	15	19	512	1069
07:45 to 08:00	553	81	47	51	14	357	1103 peak
08:00 to 08:15	561	75	59	47	19	321	1082
08:15 to 08:30	572	59	78	40	20	277	1046
08:30 to 08:45	478	46	46	34	22	252	878
08:45 to 09:00	503	82	35	64	22	329	1035
09:00 to 09:15	403	32	49	41	9	404	938
09:15 to 09:30	238	25	32	22	13	315	645
09:30 to 09:45	413	35	37	38	15	301	839
09:45 to 10:00	405	58	34	57	19	325	898
Total	6405	855	673	547	284	6315	

(SHEET 2 OF 6)

Curtis Traffic Surveys	Turning movement count			Liverpool Rd	
	160805mcl (16_408)			267	2084
Day, date	19/08/16	2 Peak Hour Volumes	183		
Location:	Hume Hy & Muir Rd		268		
Weather:	Fine	➤ Muir Rd		31	1200
Client:	McLaren Traffic Engineering				

	From Hume H'y north	From Muir	Rd	From Hume H'y south	
Time Period	through right	left	right	left through	Total vehicles Peak
15:00 to 15:15	345	67	37 45	19	287 800
15:15 to 15:30	325	75	40 40	22	295 797
15:30 to 15:45	434	82	35 50	26	300 927
15:45 to 16:00	309	67	48 96	16	<mark>234</mark> 770
16:00 to 16:15	452	62	41 86	16	<mark>267</mark> 924
16:15 to 16:30	543	83	40 50	9	320 1045 peak
16:30 to 16:45	446	65	59 62	. 12	<mark>210</mark> 854
I6:45 to I7:00	403	46 I	01 45	13	<mark>186</mark> 794
17:00 to 17:15	425	54	85 52	15	265 896
17:15 to 17:30	383	72	70 60	12	234 831
17:30 to 17:45	428	68	61 62	. 10	245 874
17:45 to 18:00	452	72	45 66	13	359 I007
18:00 to 18:15	523	75	52 70	8	301 1029
18:15 to 18:30	568	66	45 80	4	272 I035
18:30 to 18:45	541	54	41 52	6	<mark>268</mark> 962
18:45 to 19:00	526	42	31 35	10	<mark>298</mark> 942
Total	7103	1050 8	31 951	211	4341

(SHEET 3 OF 6)

Curtis Traffic Surveys	Turning moven				Liverpool Rd		
Day, date	160805mcl (16_4 19/08/16	08)		4	Peak Hour Volumes	4	
Location:	Hume Hy & Muir Rd			<u> </u>		1	Palms
Weather:	Fine					3	
Client:	McLaren Traffic E	ngineering				4	
	From Hume H'y r	orth	From Palms				
Time Period	North driveway	second driveway	third driveway	south driveway	Total vehicles	Peak	
06:00 to 06:15	C) C) (0	0	0	
06:15 to 06:30	C) C) (D	0	0	
06:30 to 06:45	C) C) (D	0	0	
06:45 to 07:00	C) C)	I	2	3	
07:00 to 07:15	C) C) :	2	0	2	
07:15 to 07:30	C) C) (D	0	0	
07:30 to 07:45	C) C) (D	1	I.	
07:45 to 08:00	1	1		I.	0	3	
08:00 to 08:15	C) C) (D	1	1	
08:15 to 08:30	1	C)	I	2	4	
08:30 to 08:45	C) C) (D	1	1	
08:45 to 09:00	1	C) 2	2	2	5 peak	
09:00 to 09:15	C) 1	(D	0	1	
09:15 to 09:30	2	c) (D	1	3	
09:30 to 09:45	1	C)	I	1	3	
09:45 to 10:00	1	c)	L	0	2	
Total	7	2) 1	I		

(SHEET 4 OF 6)

Curtis Traffic Surveys	Turning movement count			Liverpo	ol Rd	
Day, date Location: Weather: Client:	160805mcl (16_408) 19/08/16 Hume Hy & Muir Rd Fine McLaren Traffic Engineering		t v	Peak Hour \	olu me 6	I4 II Palms I8 4
	From Hume H'y north	From Pa	lms			
Time Period	North driveway second driv	eway third driv	veway south driv	eway Total vehi	cles Peak	
15:00 to 15:15	I. I.	0	I.	0	2	
15:15 to 15:30	2	1	0	0	3	
15:30 to 15:45	L. L.	0	3	0	4	
I5:45 to I6:00	2	2	I.	0	5	
16:00 to 16:15	5	4	0	0	9	
16:15 to 16:30	7	4	2	0	13	
16:30 to 16:45	L I	2	3	1	7	
16:45 to 17:00	6	2	5	0	13	
17:00 to 17:15	L I	1	2	0	4	
17:15 to 17:30	2	1	3	1	7	
17:30 to 17:45	5	3	1	1	10	
17:45 to 18:00	4	3	5	0	12	
18:00 to 18:15	3	1	2	0	6	
18:15 to 18:30	5	2	7	1	15	
18:30 to 18:45	2	2	4	2	10	
18:45 to 19:00	4	6	5	1	16 peak	
Total	51	34	44	7		

(SHEET 5 OF 6)

TRA	NS TRAF	FIC SUR	VEY	
ABN	18 434 569	5 435		
Contact	Binh S Vo			
Email	traffic@tra	ficsurvey.c	om.au	
Date	30/02/2017	2		
Weather:	Fine, max	20 deg C		
	E	т.	South	bound
	From	То	From 5 to 8 sec	> 8 sec
	7:00	7:15	18	26
	7:15	7:30	30	19
	7:30	7:45	28	18
	7:45	8:00	25	13
	8:00	8:15	29	15
	8:15	8:30	30	10
	8:30	8:45	25	14
	8:45	9:00	21	10
	9:00	9:15	22	15
	9:15	9:30	36	15
	9:30	9:45	23	22
	9:45	10:00	28	22
	16:00	16:15	16	14
	16:15	16:30	23	19
	16:30	16:45	14	20
	16:45	17:00	20	17
	17:00	17:15	23	18
	17:15	17:30	23	16
	17:30	17:45	27	15
	17:45	18:00	19	16
	18:00	18:15	18	18
	18:15	18:30	30	16
	18:30	18:45	28	15
	18:45	19:00	27	20

From	Та	Queue Length			
From	То	Number of cars			
07:00	07:05	6	16:00	16:05	2
07:05	07:10	3	16:05	16:10	4
07:10	07:15	4	16:10	16:15	3
07:15	07:20	3	16:15	16:20	7
07:20	07:25	5	16:20	16:25	3
07:25	07:30	4	16:25	16:30	6
07:30	07:35	5	16:30	16:35	4
07:35	07:40	5	16:35	16:40	5
07:40	07:45	4	16:40	16:45	4
07:45	07:50	6	16:45	16:50	3
07:50	07:55	5	16:50	16:55	2
07:55	08:00	8	16:55	17:00	4
08:00	08:05	4	17:00	17:05	5
08:05	08:10	3	17:05	17:10	5
08:10	08:15	7	17:10	17:15	7
08:15	08:20	6	17:15	17:20	6
08:20	08:25	5	17:20	17:25	4
08:25	08:30	5	17:25	17:30	6
08:30	08:35	4	17:30	17:35	5
08:35	08:40	5	17:35	17:40	6
08:40	08:45	3	17:40	17:45	6
08:45	08:50	6	17:45	17:50	5
08:50	08:55	3	17:50	17:55	3
08:55	09:00	5	17:55	18:00	3
09:00	09:05	3	18:00	18:05	5
09:05	09:10	3	18:05	18:10	3
09:10	09:15	3	18:10	18:15	4
09:15	09:20	4	18:15	18:20	3
09:20	09:25	2	18:20	18:25	2
09:25	09:30	4	18:25	18:30	2
09:30	09:35	4	18:30	18:35	1
09:35	09:40	3	18:35	18:40	2
09:40	09:45	3	18:40	18:45	1
09:45	09:50	4	18:45	18:50	2
09:50	09:55	2	18:50	18:55	3
09:55	10:00	1	18:55	19:00	1

(SHEET 6 OF 6)

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ANNEXURE C: SIDRA OUTPUT RESULTS

(Sheet 1 of 2)

MOVEMENT SUMMARY

Site: 101 [Hume & Muir AM- Existing]

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

			-					/			
Mover	nent Per	formance -	Vehic	les							
Mov	OD	Demand F	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
NorthE	ast: Hum	e Highway (N	1)								
25	T1	2217	5.0	0.581	8.9	LOS A	22.1	161.4	0.52	0.48	52.4
26	R2	295	5.0	0.548	57.8	LOS E	8.2	59.8	0.98	0.80	30.5
Approa	ach	2512	5.0	0.581	14.6	LOS B	22.1	161.4	0.58	0.52	48.3
NorthV	Vest: Muir	Road									
27	L2	234	5.0	0.300	10.8	LOS A	4.5	32.9	0.41	0.68	50.2
29	R2	161	5.0	0.234	50.3	LOS D	4.0	29.4	0.89	0.76	32.6
Approa	ach	395	5.0	0.300	26.9	LOS B	4.5	32.9	0.60	0.71	41.2
SouthV	Vest: Hun	ne Highway (S)								
30	L2	76	5.0	0.055	6.9	LOS A	0.6	4.3	0.19	0.60	53.1
31	T1	1544	5.0	0.536	21.2	LOS B	20.1	146.7	0.73	0.65	44.6
Approa	ach	1620	5.0	0.536	20.5	LOS B	20.1	146.7	0.70	0.65	44.9
All Veh	icles	4526	5.0	0.581	17.8	LOS B	22.1	161.4	0.62	0.58	46.4

MOVEMENT SUMMARY

Site: 101 [Hume & Muir PM- Existing]

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

			,			,		,			
Moven	nent Per	formance -	Vehic	les							
Mov	OD	Demand F	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
NorthE	ast: Hum	e Highway (N	1)								
25	T1	2194	5.0	0.574	8.8	LOS A	21.6	158.0	0.52	0.48	52.4
26	R2	281	5.0	0.448	54.2	LOS D	7.5	54.7	0.94	0.80	31.5
Approa	ich	2475	5.0	0.574	14.0	LOS A	21.6	158.0	0.57	0.51	48.7
NorthW	Vest: Muir	Road									
27	L2	193	5.0	0.224	8.9	LOS A	2.8	20.1	0.32	0.65	51.6
29	R2	282	5.0	0.410	52.1	LOS D	7.3	53.6	0.92	0.79	32.1
Approa	ich	475	5.0	0.410	34.6	LOS C	7.3	53.6	0.68	0.73	37.9
SouthW	Vest: Hum	ne Highway (S)								
30	L2	33	5.0	0.024	6.7	LOS A	0.2	1.6	0.18	0.58	53.2
31	T1	1263	5.0	0.461	21.9	LOS B	16.2	118.1	0.71	0.63	44.2
Approa	ich	1296	5.0	0.461	21.5	LOS B	16.2	118.1	0.70	0.63	44.4
All Vehi	icles	4245	5.0	0.574	18.6	LOS B	21.6	158.0	0.62	0.57	45.9

ANNEXURE C: SIDRA OUTPUT RESULTS

(Sheet 2 of 2)

MOVEMENT SUMMARY

Site: 101 [Hume & Muir AM- Future]

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Moven	nent Per	rformance -	Vehic	les							
Mov	OD	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
NorthE	ast: Hum	e Highway (N	1)								
25	T1	2281	5.0	0.597	9.1	LOS A	23.2	169.2	0.53	0.49	52.3
26	R2	295	5.0	0.548	57.8	LOS E	8.2	59.8	0.98	0.80	30.5
Approa	ich	2576	5.0	0.597	14.6	LOS B	23.2	169.2	0.58	0.53	48.3
NorthW	Vest: Muir	r Road									
27	L2	234	5.0	0.300	10.8	LOS A	4.5	32.9	0.41	0.68	50.2
29	R2	161	5.0	0.234	50.3	LOS D	4.0	29.4	0.89	0.76	32.6
Approa	ich	395	5.0	0.300	26.9	LOS B	4.5	32.9	0.60	0.71	41.2
SouthV	Vest: Hun	ne Highway (S)								
30	L2	76	5.0	0.055	6.9	LOS A	0.6	4.3	0.19	0.60	53.1
31	T1	1544	5.0	0.536	21.2	LOS B	20.1	146.7	0.73	0.65	44.6
Approa	ich	1620	5.0	0.536	20.5	LOS B	20.1	146.7	0.70	0.65	44.9
All Veh	icles	4591	5.0	0.597	17.8	LOS B	23.2	169.2	0.63	0.58	46.4

MOVEMENT SUMMARY

Site: 101 [Hume & Muir PM- Future]

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

								,			
Mover	ment Per	formance -	Vehic	les							
Mov	OD	Demand F	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
NorthE	ast: Hum	e Highway (N	1)								
25	T1	2220	5.0	0.581	8.9	LOS A	22.1	161.3	0.52	0.48	52.4
26	R2	281	5.0	0.448	54.2	LOS D	7.5	54.7	0.94	0.80	31.5
Approa	ach	2501	5.0	0.581	14.0	LOS A	22.1	161.3	0.57	0.52	48.7
NorthV	Vest: Muir	Road									
27	L2	193	5.0	0.224	8.9	LOS A	2.8	20.1	0.32	0.65	51.6
29	R2	282	5.0	0.410	52.1	LOS D	7.3	53.6	0.92	0.79	32.1
Approa	ach	475	5.0	0.410	34.6	LOS C	7.3	53.6	0.68	0.73	37.9
South	Vest: Hun	ne Highway (S)								
30	L2	33	5.0	0.024	6.7	LOS A	0.2	1.6	0.18	0.58	53.2
31	T1	1263	5.0	0.461	21.9	LOS B	16.2	118.1	0.71	0.63	44.2
Approa	ach	1296	5.0	0.461	21.5	LOS B	16.2	118.1	0.70	0.63	44.4
All Veh	nicles	4272	5.0	0.581	18.6	LOS B	22.1	161.3	0.62	0.57	45.9